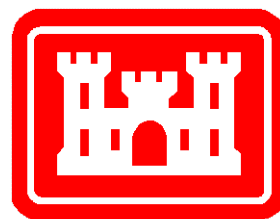


OSM VALLEY FILL STUDY

SAMPLES MINE VALLEY FILL #2 AOC+ CONDITIONS



Appalachian
Regional
Coordinating
Center



**US Army Corps
of Engineers**
Pittsburgh District

OCTOBER 2000

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GENERAL

The intent of this study was to determine the effect on storm runoff by changes to topography, soils, land use, vegetation, etc, caused by mountain top removal / valley fill surface coal mining operations. The changes to the 10 and 100 year flows and water surface elevations were determined and compared for the premining, post mining and AOC+ (Approximate Original Contour Plus) conditions.

This report covers the results from the AOC+ conditions only. The results of the study for premining and post mining have been previously reported. They will be included in this report by reference and by inclusion in the "HYDROLOGIC AND HYDRAULIC MODEL RESULTS" section.

The Samples Mine Valley Fill SH-2, located in the headwaters of the Seng Creek watershed in Boone County, West Virginia, was selected as the study site. The determination of the effects of changes to this drainage area represents a classic ungaged watershed study. The Seng Creek watershed is ungaged and no historic hydrologic information is available.

Corps of Engineers personnel from the Pittsburgh District (Mark Zaitsoff, Dennis McCune, Elizabeth Rodriguez) and Office of Surface Mining (OSM) personnel (Dan Rahnema) visited the site.

Discussions were held to determine the methods of analysis that could be used to achieve the required results. Since great changes occur to the drainage area from pre to AOC+ conditions, the method of analysis needed to be able to subdivide it and model the changed areas as appropriate. Those involved concurred that the HEC-HMS (Hydrologic Modeling System) and HEC-RAS (River Analysis System) models would provide the methods of analysis and results needed for the study.

A HEC-HMS (version 1.1) rainfall runoff model was used to evaluate the changes in flow magnitude. The runoff curve number (CN) method developed by the Soil Conservation Service (SCS) (now National Resource Conservation Service, NRCS) was used to determine the rainfall losses and the transformation from rainfall excess to runoff. This method has the advantage over regional parameter methods of rainfall-runoff determination of being based on observable physical properties of the watershed and of being able to model great changes in the runoff characteristics of the watershed.

A HEC-RAS (version 2.2) hydraulic model was used to provide peak flow timing and routing input to the HEC-HMS hydrologic model. Flows generated by the hydrology model were input to the hydraulic model until the input and output from both models were consistent. The HEC-RAS model was then used to determine the changes in water surface elevation.

Topographic maps, aerial photographs and survey cross sections were used to formulate these hydrologic and hydraulic models.

This study was conducted under interagency agreement number 143868-IA98-1244, entitled "Model Analysis of Potential Downstream Flooding as a Result of Valley Fills and Large-Scale Surface Coal Mining Operations in Appalachia", between the Office of Surface Mining Reclamation and Enforcement and the U.S. Army Corps of Engineers. The Samples Mine Valley Fill #2 was the second site studied. The other three were the Samples Mine Valley Fill #1, #1 and 2 combined and the Hobet Mine Westridge Valley Fill in Lincoln County, WV. Results from these other sites have been reported separately. The study was initiated 24 September 1998.

DESCRIPTION OF HYDROLOGIC AND HYDRAULIC MODELS

Drainage Area

The Samples Mine Valley Fill SH-2 is located approximately 25 miles southeast of Charleston, WV, on the eastern side of Boone County on the boundaries with Kanawha and Raleigh Counties, WV. It is located in the headwaters of the Seng Creek (tributary to the Big Coal and Kanawha Rivers) watershed. The valley fill drainage area occupies a 0.6 square mile (10%) unnamed tributary near the upstream end of the 5.55 square mile Seng Creek watershed.



Precipitation

Precipitation depths were determined using the National Weather Service publications HYDRO35 and Technical Paper 40 (TP40). HYDRO 35 provides maps of rainfall depths for 5, 15 and 60 minute durations, and 2 and 100 year frequencies. Equations are provided to calculate the precipitation depths for other frequencies. TP40 provides maps of precipitation depths for 2, 3, 6, 12 and 24 hour durations, and 1 to 100 year frequencies.

The Samples Mine is located on the eastern side of Boone County, WV, and that location was used to determine the precipitation depths. The following table shows the precipitation depths determined from HYDRO 35 and TP40 for the study area:

Duration	Frequency [YR]	
	10	100
	Depth [IN]	
5 MIN	0.54	0.74
15 MIN	1.09	1.57
1 HR	1.86	2.70
2 HR	2.38	3.44
3 HR	2.68	3.76
6 HR	3.05	4.44
12 HR	3.53	5.06
24 HR	3.98	5.65

These values were used for the premining, post and AOC+ mining conditions.

Soil Types

The Boone County, WV, soil survey was used to determine the soil types located in the study area.

The Seng Creek watershed is contained within the Dekalb-Pineville-Guyandotte general soil unit. The soils within this unit are described as "very steep, well drained soils that formed mainly in material weathered from sandstone; on mountainous uplands". The various soil types within this unit are the Cedar creek-Rock outcrop (CgF), Dekalb-Pineville-Guyandotte association (DPF), Kaymine-Cedar creek-Dekalb (KmF), Kaymine-Rock outcrop complex (KrF), and Lily-Dekalb complex (LdE). The soil survey provides information on the detailed make up of the soil types, giving such information as component soil types, impervious area, etc.

The soil type subareas were traced onto the USGS topographic or regraded drainage maps for the premining, postmining and AOC+ conditions; the areas of each soil type within the runoff subareas were determined by planimetry.

SCS Runoff Curve Numbers

The SCS runoff curve number (CN) method was used to convert precipitation depth into runoff excess. The curve number method is based on observable physical properties (soil and cover) of the runoff subareas.

A hydrologic soil group (HSG) characterizes the soil properties. The soil survey provides information on the detailed make up of the various soil types, making it possible to classify their component soils into HSG A (low runoff potential and high infiltration rates) through HSG D (high runoff potential and very low infiltration rates).

The cover takes into account the land use, vegetation type, surface treatment, etc.

The curve number is determined by the combination of the component soil types and cover. Curve numbers were selected from the tables published and provided by the SCS. It is possible to calculate areal weighted curve numbers for the overall soil types and each runoff subarea.

The curve number is also used to calculate the initial abstraction (all losses before runoff begins) for each runoff subarea. This initial abstraction (I_a) is defined as 20% of the maximum available retention capacity of the soil after the runoff begins.

Time of Concentration and Lag

The time of concentration (T_c) of each runoff subarea is the amount of time that it takes for runoff to travel from the hydraulically most distant point to the outlet. It is the sum of the travel times (T_t) through the components of the runoff system.

The SCS method provides procedures for computing three travel time components for the time of concentration calculations: 1) sheet flow, 2) shallow concentrated flow, and 3) open channel flow.

Sheet flow is the runoff that occurs over the surface of the ground prior to becoming concentrated into small gullies. It is limited, by definition in the SCS method, to a maximum of 300 feet from the most upstream drainage divide. Shallow concentrated flow occurs from the end of sheet flow until the runoff enters a channel, by definition a stream shown on a USGS map. Appropriate changes in slopes were incorporated into the calculations of sheet and shallow concentrated flows. HEC-HMS computed values for the 10 and 100 year flows were input to the HEC-RAS hydraulic model of the valley fill drainage area to provide travel times for the channel flow component. The undisturbed portion of the unnamed tributary was used for the open channel flow component for the subareas below the valley fill operation.

The sum of the three travel time components is the time of concentration for a runoff subarea.

Several flow routes were considered when calculating the time of concentration for each runoff subarea. The different routes were selected to maximize the effect of each of the three components on the time of concentration. They maximized the flow distances for each component; the flow route giving the greatest time of concentration was selected.

The lag (L) is defined as the time from the center of mass of the excess rainfall to the peak of the calculated hydrograph. The lag is defined and calculated by the SCS method as 60% of the time of concentration.

Base Flow

A base flow of 2 CFS/SM was adopted for each runoff subbasin. Since the base flow contribution to the volume and peak discharge is minor, the recession constant and threshold were estimated in the HEC-HMS model to be 1 (no recession) and 0 CFS, respectively. This gives a constant base flow value of 2 CFS/SM during the entire flow hydrograph.

Routing Reaches

A HEC-RAS hydraulic model was used to determine the required inputs for the hydrologic routing. This model was formulated using survey cross sections and topographic map information. Channel reach lengths and slopes were estimated from the OSM 1:4,800 scale maps that had a contour interval of 20'. Cross section geometry, channel roughness, reach lengths, energy slopes and average travel times from the HEC-RAS model were used as input to the Muskingum-Cunge and Lag routing methods in the HEC-HMS models.

The HEC-HMS hydrology models route upstream flows through intervening runoff subareas, then combine routed flows and local runoff at the downstream end of the routing reaches. This hydrologic routing provides the translation of the

flow hydrograph along the channels and the timing and attenuation that reflect the storage characteristics of the channel and overbank sections of the routing reaches.

The HEC-RAS model was formulated to add in the local runoff in five increments through each routing reach, increasing the channel flow progressing downstream. The HEC-HMS model results show that there was little change in the routed flow through the routing reaches, so this assumption of local flow increasing along a routing reach was not affected by routing considerations.

AOC+ CONDITIONS

Drainage Areas

The AOC+ mining condition drainage area was delineated on a 1:4,800 scale regraded drainage map provided by the Knoxville Field Office of OSM. The AOC+ mining condition drainage area encompasses 0.50 square miles.

The drainage area was divided into sixteen runoff subareas to define the AOC+ mining condition. These subareas were selected to define tributary areas created by sediment and diversion ditches in the regrading plan and the hydrologic routing reaches connecting them. The downstream end of the drainage area is relatively unchanged from premining conditions; the unchanged land use, soil types and tributary justified further subdivision. The regraded drainage map shows that the AOC+ mining land use is reclaimed valley fill and backstack areas for 56% of the drainage area.

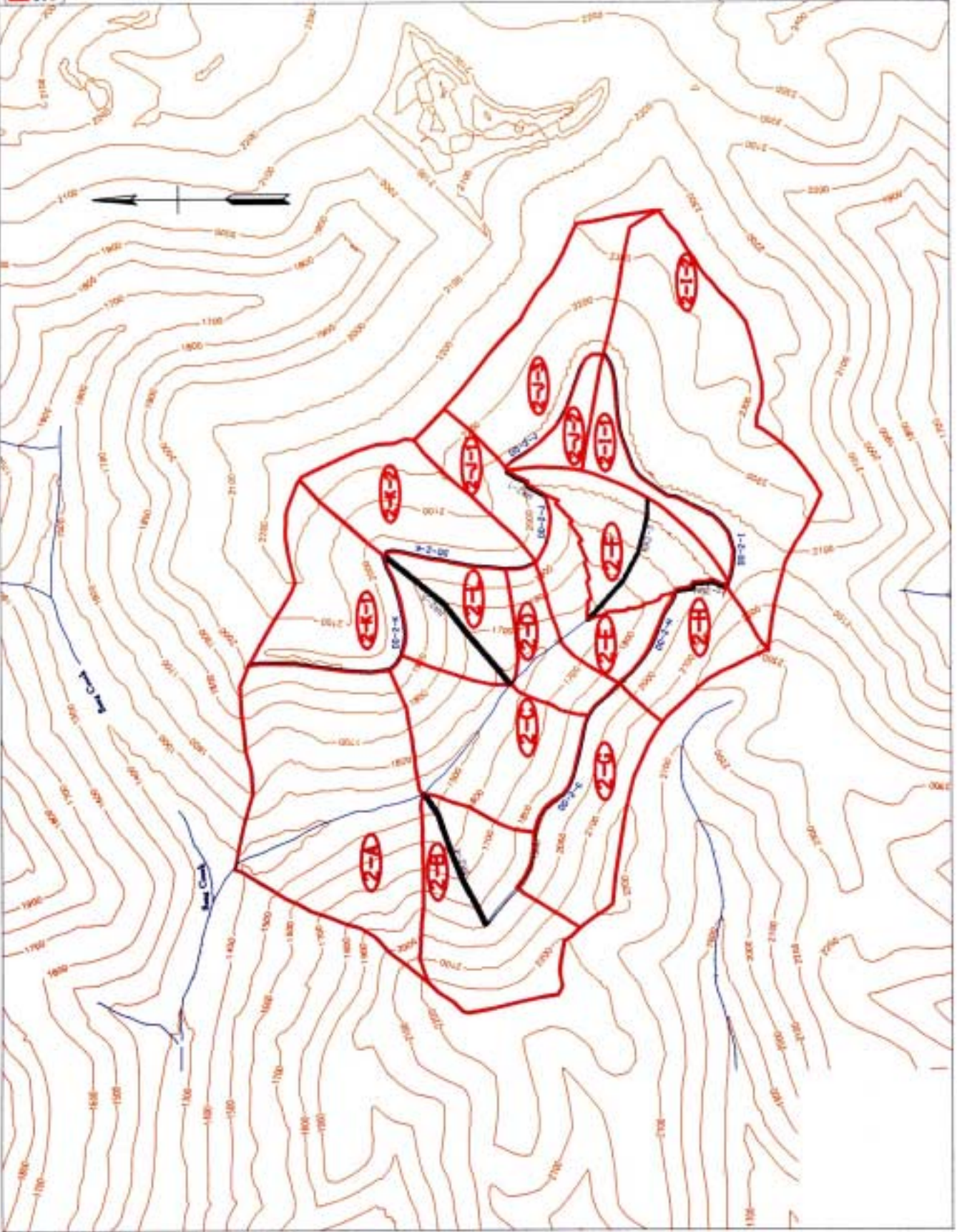
The regraded drainage plan used sediment and diversion ditches to create four tributary areas. These four tributary areas were: 1) below the valley fill, 2) the valley fill area, 3) flows diverted around the left side of the valley fill, and 4) flows diverted around the right side of the valley fill. The following table shows the runoff subareas for the AOC+ mining condition:

Runoff Subarea	Description	Area		
		[ACRES]	[MI ²]	[%]
2-A	Most downstream area	50.32	0.08	15.8
2-C	Subarea below valley fill	24.26	0.04	7.6
2-D	Right bank tributary	8.96	0.01	2.8
2-E	Subarea below valley fill	11.02	0.02	3.5
2-F	Face of valley fill	17.25	0.03	5.4
2-B	Downstream left diversion area	27.15	0.04	8.5
2-G	Middle left diversion area	19.21	0.03	6.0
2-H	Upstream left diversion area	12.01	0.02	3.8
2-I-1	Left top of valley fill	7.17	0.01	2.2
2-I-2	Left backstack area	50.53	0.08	15.8
2-L	Downstream right diversion area	14.84	0.02	4.6
2-K-1	Downstream right diversion area	20.36	0.03	6.4
2-K-2	Middle right diversion area	18.98	0.03	5.9
2-J-1	Upstream right diversion area	8.85	0.01	2.8
2-J-2	Right top of valley fill	4.61	0.01	1.4
2-J-3	Right backstack area	23.86	0.04	7.5
Total		319.38	0.50	100

This area represents a 9% decrease from pre to AOC+ mining conditions.

Plate 1 shows the runoff subareas.

SCALE: 1" = 1000'



Soil Types and SCS Runoff Curve Numbers

The regraded drainage map shows the area that was covered by the valley fill and regraded backstacks. These areas were considered to be reclaimed surface mine (RSM) areas.

The following table shows the soil types and their percent distribution within the runoff subareas for the AOC+ mining condition:

Runoff Subarea	Soil Type					
	CgF	DPF	KmF	KrF	LdE	RSM
	Percent Distribution					
2-A	0.7	87.0		11.1	1.2	
2-C		87.8		12.2		
2-D		100				
2-E		100				
2-F						100
2-B		62.9			37.1	
2-G		4.5			14.8	80.7
2-H		16.3	7.0			76.7
2-I-1						100
2-I-2	23.6	1.3			26.5	48.6
2-L		97.0		3.0		
2-K-1						100
2-K-2						100
2-J-1						100
-J-2						100
2-J-3	9.0				7.8	83.2
Total	2.1	34.8	0.4	1.6	5.5	55.6

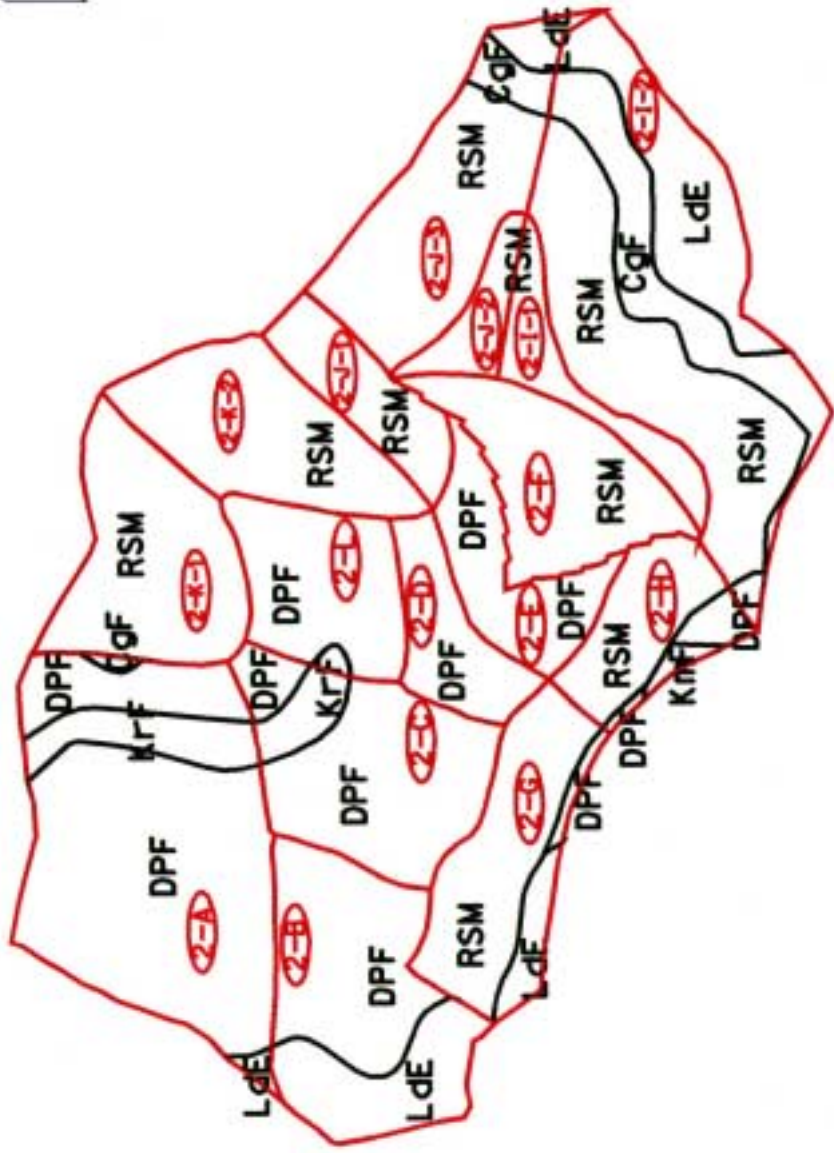
Plate 2 shows the soil type subareas.

This table shows that reclaimed surface mine areas make up the majority (56%) of the land use in the drainage area.

The land use for the undisturbed portion of the valley fill drainage area is wooded with a fair hydrologic condition due to its disturbance by previous logging and surface mining activity.

The following table shows the results of the weighted curve number calculations for the AOC+ mining condition:

SCALE: 1" = 1000'



Runoff Subarea	Weighted CN	% Impervious	I _a [IN]
2-A	67	1.8	0.99
2-C	67	1.8	0.99
2-D	66		1.03
2-E	66		1.03
2-F	75		0.67
2-B	66		1.03
2-G	73		0.74
2-H	73		0.74
2-I-1	75		0.67
2-I-2	72	3.5	0.78
2-L	66	0.4	1.03
2-K-1	75		0.67
2-K-2	75		0.67
2-J-1	75		0.67
2-J-2	75		0.67
2-J-3	74	1.3	0.70

Time of Concentration and Lag

The regraded drainage map was used to define the distance for sheet flow. The runoff was considered to have concentrated once it encountered a road or bench and continued to flow downslope to a sediment ditch. The sediment ditches were considered the open channel portion of the flow components.

The following table shows the results of the time of concentration and lag calculations for the AOC+ mining condition:

Runoff Subarea	Frequency [YR]			
	10		100	
	Time of Concentration	Lag	Time of Concentration	Lag
	[MIN]			
2-A	19	11	18	11
2-C	17	10	17	10
2-D	8	5	7	4
2-E	10	6	10	6
2-F	12	7	11	6
2-B	20	12	19	11
2-G	15	9	13	8
2-H	12	7	11	7
2-I-1	41	25	38	23
2-I-2	36	21	33	20
2-L	8	5	7	4
2-K-1	29	17	27	16
2-K-2	17	10	15	9
2-J-1	13	8	13	8
2-J-2	29	17	28	17
2-J-3	24	14	22	13

Base Flow

The AOC+ mining condition base flow values were as follows:

Runoff Subarea	Area [MI ²]	Base Flow [CFS]
2-A	0.08	0.16
2-C	0.04	0.08
2-D	0.01	0.03
2-E	0.02	0.03
2-F	0.03	0.05
2-B	0.04	0.08
2-G	0.03	0.06
2-H	0.02	0.04
2-I-1	0.01	0.02
2-I-2	0.08	0.16
2-L	0.02	0.05
2-K-1	0.03	0.06
2-K-2	0.03	0.06
2-J-1	0.01	0.03
2-J-2	0.01	0.01
2-J-3	0.04	0.07

Routing Reaches

The valley fill drainage area was divided into sixteen runoff subareas to model the AOC+ mining condition. Twelve reaches connected the runoff subareas and routed the flows through the drainage area.

Two methods of hydrologic routing were used to route the runoff flows through the drainage area. The Lag method was used for channels with slopes greater than 10% (flumes and natural drains); the amount of lag was taken as the average travel time through the reach from the HEC-RAS model. Since these channels have little if any storage they were modeled to translate the flow hydrograph with no attenuation. The Muskingum-Cunge method was used to route the runoff flows through the flatter sloped (0.5%) sediment and diversion ditches. This method has the advantage over others of using physically based parameters that can be modified to represent changes to the watershed conditions.

HYDROLOGIC AND HYDRAULIC MODEL RESULTS

The HEC-HMS hydrology models were formulated to calculate the outflow from the Valley Fill #2 drainage area at the downstream permit limit.

The HEC-RAS hydraulic model was formulated to calculate the corresponding stages. Survey sections were taken and the undisturbed unnamed tributary channel was modeled. The flows from the HEC-HMS model were used to perform the backwater analysis.

The following table shows the 10 and 100 year flows and water surface elevations:

Frequency [YR]	Pre Mining		Post Mining		AOC+	
	Flow [CFS]	Elevation [FT NGVD]	Flow [CFS]	Elevation [FT NGVD]	Flow [CFS]	Elevation [FT NGVD]
10	293	1333.1	302	1333.1	295	1333.1
100	664	1334.5	671	1334.5	658	1334.5

YR = Years

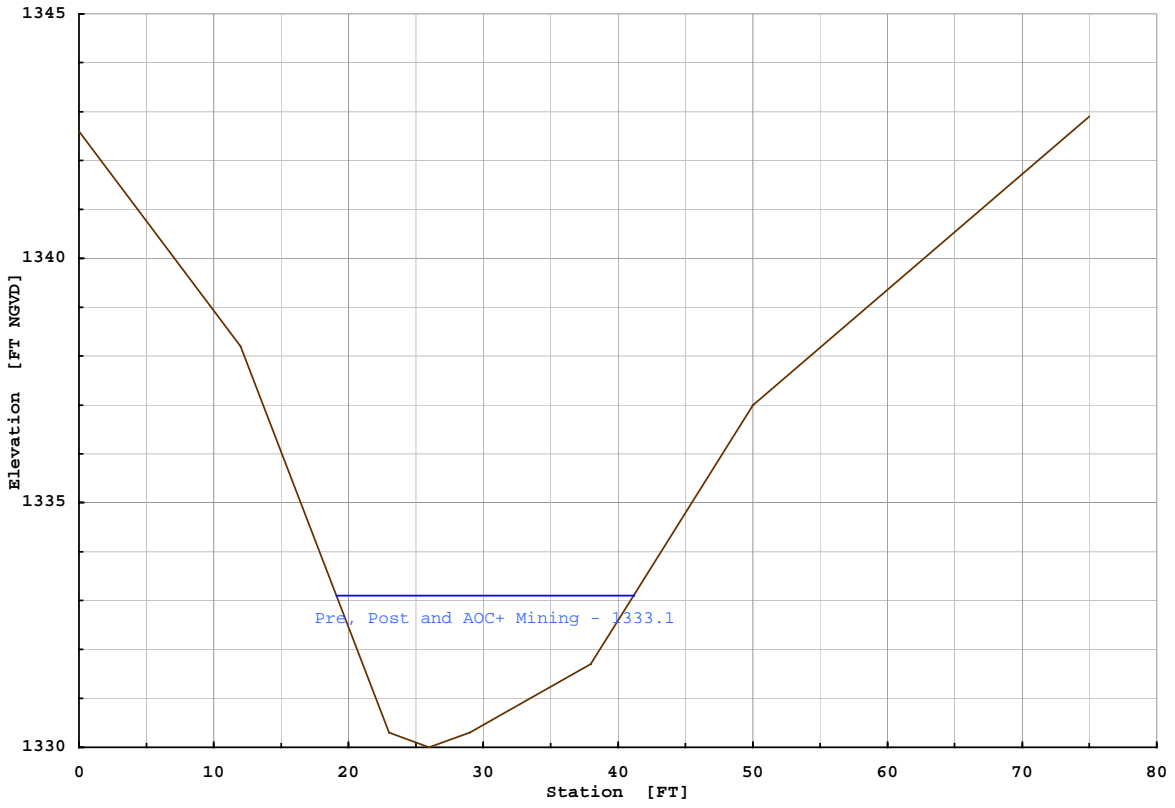
CFS = Cubic Feet per Second

FT NGVD = Feet above National Geodetic Vertical Datum

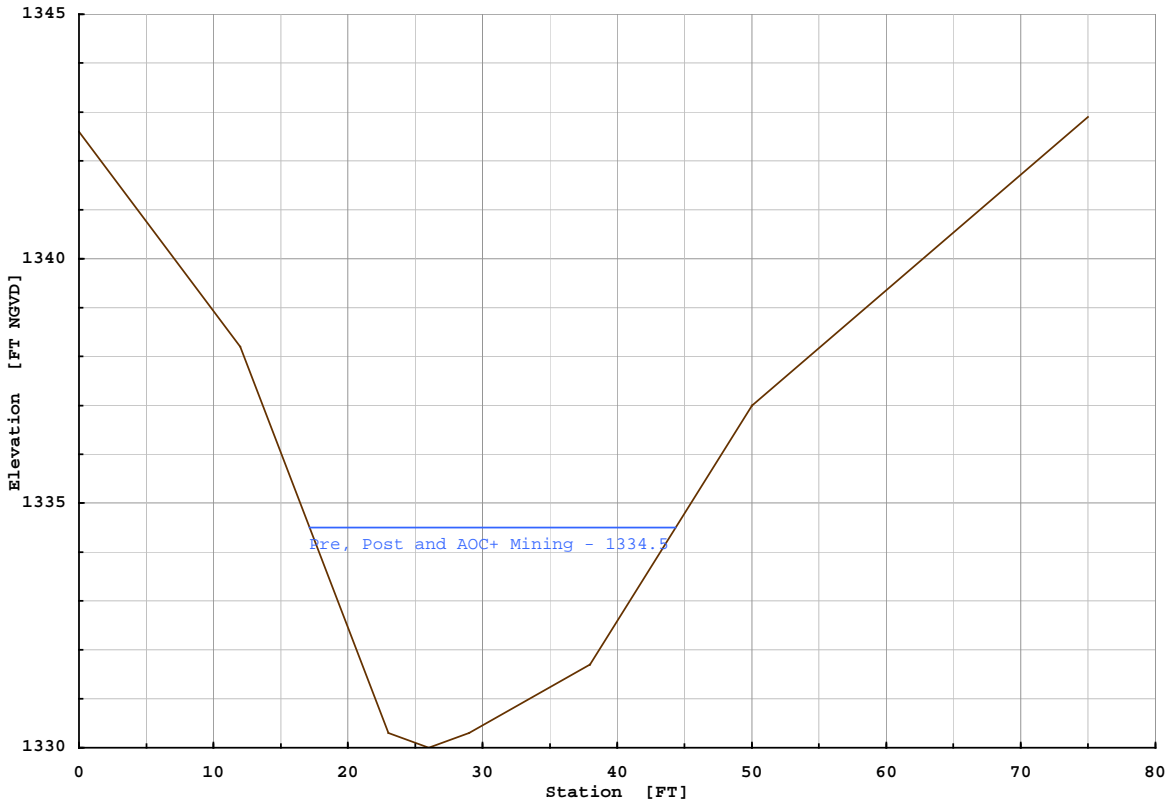
These results show a 3-1% (10-100 YR) increase in discharge from premining conditions after the area is reclaimed in the post mining conditions. There are no stage increases from pre to post mining conditions. Alternatively, the AOC+ conditions would cause a 1% (10 YR) increase and 1% (100 YR) decrease in discharge with no change in stage from premining condition.

The following cross section shows comparisons of the water surfaces for each condition.

COMPARISON OF STAGES FOR 10 YEAR FLOW



COMPARISON OF STAGES FOR 100 YEAR FLOW



CONCLUSIONS

1. The SCS, HEC-HMS and HEC-RAS methods are appropriate for computing flows and stages from a valley fill operation.
2. The information typically contained in a permit application is suitable for hydrologic and hydraulic analysis. Some interpretation of the information, aerial photos and maps is required.
3. Required additional information about soil types is available from soil surveys.
4. Field views are required to determine the type and extent of cover for HEC-HMS, to verify drainage routes, etc.
5. Field surveys are required to determine channel size and compute stages in HEC-RAS.
6. Subdivision of the valley fill area by soil type, slopes, etc, is required to model the runoff characteristics of each subarea. Subdivision will increase the complexity of the hydrologic and hydraulic models.
7. It is not possible to generalize the impacts of changes to the drainage area on the discharge. Changes to the topography, soils, land use, vegetation will cause corresponding changes to the discharge. Changes to the flow paths will affect the discharge by changing the runoff time of concentration, flow routing times and hydrograph combination.
8. Differences in stages are very site specific and may depend on conditions in receiving streams. Stage differences cannot be translated up or down stream away from the computed location and results should not be generalized. Unchanged watershed and channel downstream of a valley fill operation may tend to return stages to the premining condition.
9. This study shows a 3-1% (10-100 YR) increase in discharge from premining conditions after the area is reclaimed in the post mining conditions. There are no stage increases from pre to post mining conditions. Alternatively, the AOC+ conditions would cause a 1% (10 YR) increase and 1% (100 YR) decrease in discharge with no change in stage from premining condition.

RECOMMENDATIONS

1. The site should be analyzed with a mature growth of trees covering all or part of the drainage area to represent a future condition. Incremental analysis of increasing tree cover should not be undertaken.

2. Valley fill operations should be sized and located to minimize their impacts.

3. Recording streamflow and rainfall gages should be installed and maintained in a valley fill area from before mining begins until after the area is reclaimed. Data logger type streamflow gages should be installed at good hydraulic control points and be set to record at five minute intervals. Tipping bucket type rainfall gages should be located to capture representative rainfall amounts. A formal maintenance and data retrieval/reduction plan should be established. Analysis of actual rainfall/runoff relations should be conducted.

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